

1 General equalities

Spherical symmetric part of tensor: (Captures *change in volume* for ε)

$$\mathbf{A}^s = \frac{1}{3} \text{tr}(\mathbf{A}) * \mathbf{I} \quad (1)$$

Deviatoric part of tensor: (Captures *change in shape* for ε)

$$\mathbf{A}^d = \mathbf{A} - \mathbf{A}^s = \mathbf{A} - \frac{1}{3} \text{tr}(\mathbf{A}) * \mathbf{I} \quad (2)$$

Gauss Theorem:

$$\int_V \nabla \mathbf{v} dV = \int_S \mathbf{v} \cdot \mathbf{n} dS \quad (3)$$

Angle between vectors:

$$\mathbf{v} \cdot \mathbf{w} = \|\mathbf{v}\| \|\mathbf{w}\| \cos(\angle \mathbf{vw}) \quad (4)$$

ABC-formula:

$$x = \frac{-b \pm \sqrt{b^2 - 4ac}}{2a} \quad (5)$$

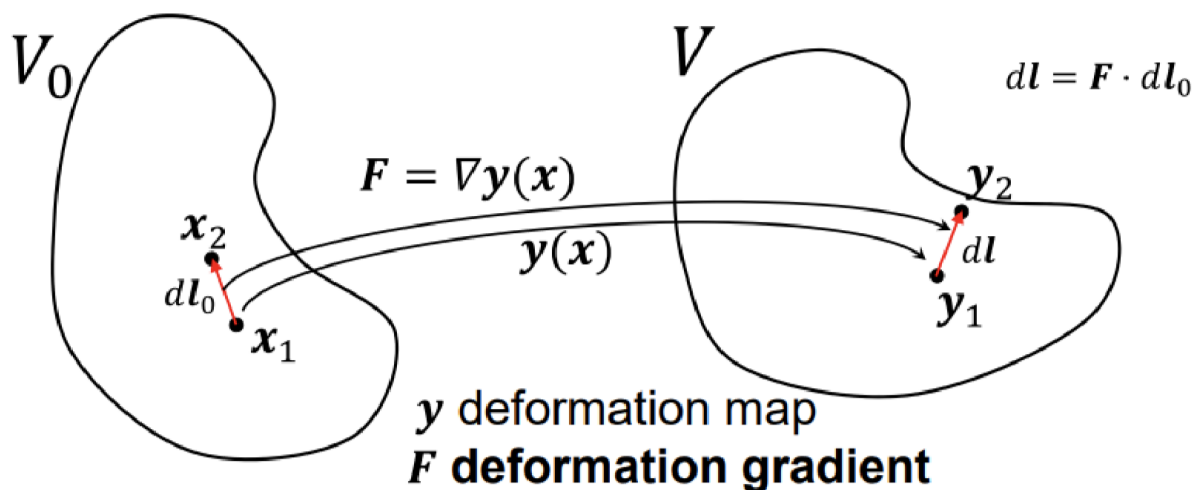
Rotation:

$$\mathbf{R} = \begin{bmatrix} \cos(\alpha) & \sin(\alpha) & 0 \\ \sin(\alpha) & -\cos(\alpha) & 0 \\ 0 & 0 & 1 \end{bmatrix} \quad \det(\mathbf{R}) = 1, \mathbf{R}^T = \mathbf{R}^{-1} \quad (6)$$

2 Small strain:

“Reference” configuration

“Current” configuration



$$\mathbf{F} = \nabla \mathbf{y} = \frac{\partial \mathbf{y}}{\partial \mathbf{x}} \quad (7)$$

Displacement of point

$$\mathbf{u}(\mathbf{x}) = \mathbf{y}(\mathbf{x}) - \mathbf{x} \quad (8)$$

Displacement gradient:

$$\nabla \mathbf{u} = \nabla \mathbf{y} - \mathbf{I} = \varepsilon + \omega \quad (9)$$

Strain:

$$\varepsilon = \frac{1}{2} * (\nabla \mathbf{u} + \nabla \mathbf{u}^T) \quad (10)$$

Rotation:

$$\omega = \frac{1}{2} * (\nabla \mathbf{u} - \nabla \mathbf{u}^T) \quad (11)$$

Only when $\|u\| = \sqrt{u : u} \ll 1$!

2.1 Invariants:

if eigenvalues of ε are given by ϵ_i .

$$I_{\mathbf{A}} = \text{tr}(\mathbf{A}) \quad (12)$$

$$II_{\mathbf{A}} = \epsilon_1 * \epsilon_2 + \epsilon_2 * \epsilon_3 + \epsilon_1 * \epsilon_3 \quad (13)$$

$$III_{\mathbf{A}} = \det(\mathbf{A}) = \epsilon_1 * \epsilon_2 * \epsilon_3 \quad (14)$$

3 Kinematics at large deformation

Nanson's Formula (change in area)

$$\mathbf{n} dA = \det(\mathbf{F}) \mathbf{F}^{-T} \cdot \mathbf{n}_0 dA_0 \quad (15)$$

Change in volume:

$$\det(\mathbf{F}) = \frac{dV}{dV_0} \quad (16)$$

Relative change in volume:

$$\text{tr}(\varepsilon) = \frac{dV - dV_0}{dV_0} \quad (17)$$

Right Cauchy-Green deformation:

$$\mathbf{C} = \mathbf{F}^T \cdot \mathbf{F} \quad (18)$$

$\sqrt{\mathbf{C}} = \mathbf{U}$ Pure deformation!

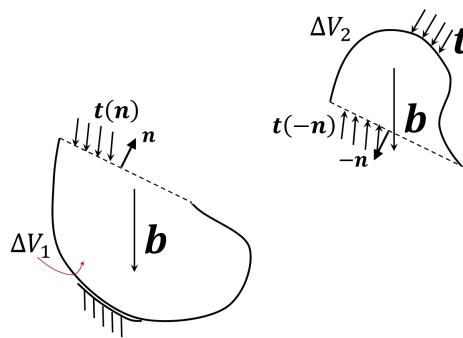
Polar decomposition ((Rotation) · (Pure deformation)):

$$\mathbf{F} = \mathbf{R} \cdot \mathbf{U} \quad (19)$$

Find $\mathbf{U}$ using $\sqrt{\mathbf{C}} = \mathbf{P} \mathbf{D}^{1/2} \mathbf{P}^{-1}$, ($\mathbf{P}$ is eig. vector matrix of $\mathbf{C}$, $\mathbf{D}$ is eig. value matrix of ($\mathbf{C}$)). Find $\mathbf{R} = \mathbf{F} \cdot \mathbf{U}^{-1}$

Green Langrange Strain:

$$\mathbf{E} = \frac{1}{2}(\mathbf{C} - \mathbf{I}) \quad (20)$$



Cauchy's equation:

$$\mathbf{t}(\mathbf{n}) = \sigma \cdot \mathbf{n} \iff t_i = \sigma_{ij} n_j \quad (21)$$

The stress tensor (σ) used can be calculated using:

$$\sigma = \mathbf{t}(\mathbf{n}) \otimes \mathbf{n} \quad (22)$$

The normal stress (σ) is calculated by projecting the vector $\mathbf{t}$ along $\mathbf{n}$ (vector initially chosen)

$$\sigma = \mathbf{t}(\mathbf{n}) \cdot \mathbf{n} \quad (23)$$

The shear stress (τ) is calculated by projecting the vector $\mathbf{t}$ along a vector along its plane ($\mathbf{s}$).

$$\tau = \mathbf{t}(\mathbf{n}) \cdot \mathbf{s} \quad (24)$$

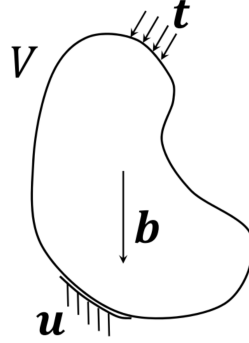
Deviatoric stress:

$$\sigma^d = \sigma - \frac{\text{tr}(\sigma)}{3} \mathbf{I} \quad (25)$$

Hydrostatic stress:

$$p\mathbf{I} = \frac{\text{tr}(\sigma)}{3} \mathbf{I} \quad (26)$$

4 Stress in small deformations



Balance of forces: (equilibrium of momentum)

$$\int_V \mathbf{b} dV + \int_S \mathbf{t} dS \iff \mathbf{b} + \text{div}(\sigma) = 0 \iff b_i + \sigma_{ij,j} = 0 \quad (27)$$

Equilibrium of moment of moments:

$$\sigma = \sigma^T \quad (28)$$

$$\delta W^{ext} = \int_v \mathbf{b} \cdot \delta \mathbf{u} dV + \int_s \mathbf{t} \cdot \delta \mathbf{u} dS \quad (29)$$

$$\delta W^{int} = \int_V \delta \epsilon : \sigma dV \quad (30)$$

$$\delta W^{ext} = \delta W^{int} \quad (31)$$

5 Elasticity in small deformations

$$\sigma = \mathbb{C} : \epsilon \quad (32)$$

Internal stored energy *density* (i.e. per volume): (Hooke's law)

$$\psi = \frac{1}{2} \sigma : \epsilon = \frac{1}{2} \epsilon : \mathbb{C} : \epsilon \quad (33)$$

Stress in a material is the change in internal stored energy over change in strain:

$$\sigma = \frac{\partial \psi}{\partial \epsilon} \quad (\iff \sigma_{ij} = \frac{\partial \psi}{\partial \epsilon_{ij}}) \quad (34)$$

Isotropic stiffness tensor (symmetries due to isotropy)

$$\mathbb{C} = \lambda \mathbf{I} \otimes \mathbf{I} + 2\mu \mathbb{I}^s \quad (35)$$

With $\mathbb{I}^s$ being the 4th order symmetric unit tensor.

$$\mathbb{C}_{ijhk} = \lambda \delta_{ij} \delta_{hk} + \mu (\delta_{ih} \delta_{jk} + \delta_{ik} \delta_{jh}) \quad (36)$$

$$[\sigma] = [\mathbb{C}] : [\varepsilon] = \begin{bmatrix} \sigma_{11} \\ \sigma_{22} \\ \sigma_{33} \\ \sigma_{12} \\ \sigma_{13} \\ \sigma_{23} \end{bmatrix} = \begin{bmatrix} \lambda + 2 \cdot \mu & \lambda & \lambda & 0 & 0 & 0 \\ \lambda & \lambda + 2 \cdot \mu & \lambda & 0 & 0 & 0 \\ \lambda & \lambda & \lambda + 2 \cdot \mu & 0 & 0 & 0 \\ 0 & 0 & 0 & \mu & 0 & 0 \\ 0 & 0 & 0 & 0 & \mu & 0 \\ 0 & 0 & 0 & 0 & 0 & \mu \end{bmatrix} \begin{bmatrix} \varepsilon_{11} \\ \varepsilon_{22} \\ \varepsilon_{33} \\ 2\varepsilon_{12} \\ 2\varepsilon_{13} \\ 2\varepsilon_{23} \end{bmatrix} \quad (37)$$

$$\sigma = \lambda tr(\varepsilon) \mathbf{I} + 2\mu \varepsilon \quad (38)$$

4th order compliance tensor ($\mathbb{S}$):

$$\varepsilon = \mathbb{S} : \sigma \quad (39)$$

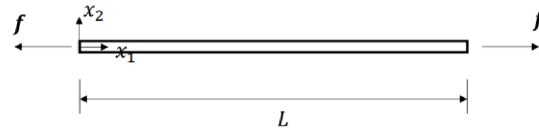
$$\mathbb{S} = -\frac{\lambda}{2\mu(3\lambda + 2\mu)} \mathbf{I} \otimes \mathbf{I} + \frac{1}{2\mu} \mathbb{I}^s \quad (40)$$

$$\varepsilon = -\frac{\lambda}{2\mu(3\lambda + 2\mu)} tr(\sigma) \mathbf{I} + \frac{1}{2\mu} \sigma \quad (41)$$

	(E, ν)	(K, μ)	(E, μ)
E	E	$\frac{9\mu K}{3K + \mu}$	E
ν	ν	$\frac{3K - 2\mu}{2(3K + \mu)}$	$\frac{E - 2\mu}{2\mu}$
G, μ	$\frac{E}{2(1 + \nu)}$	μ	μ
K	$\frac{E}{3(1 - 2\nu)}$	K	$\frac{\mu E}{3(3\mu - E)}$
λ	$\frac{E\nu}{(1 + \nu)(1 - 2\nu)}$	$\frac{3K - 2G}{3}$	$\frac{\mu((E - 2\mu))}{3\mu - E}$

6 Beam Theory

6.1 Uniaxial tension



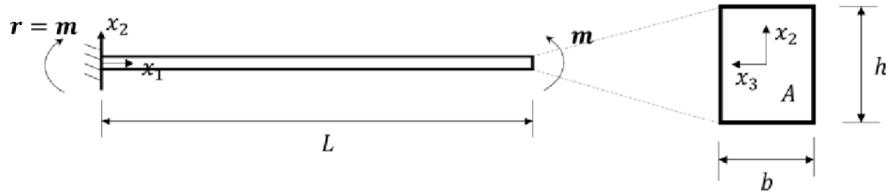
Stress tensor:

$$\sigma = \frac{\mathbf{f}}{A} \mathbf{e}_1 \otimes \mathbf{e}_1 \quad (42)$$

using $\varepsilon = -\nu \text{tr}(\sigma) \mathbf{I} + (1 + \nu) \sigma$:

$$\varepsilon = \frac{\mathbf{f}}{EA} \mathbf{e}_1 \otimes \mathbf{e}_1 - \frac{\nu}{E} \frac{\mathbf{f}}{A} \mathbf{e}_2 \otimes \mathbf{e}_2 - \frac{\nu}{E} \frac{\mathbf{f}}{A} \mathbf{e}_3 \otimes \mathbf{e}_3 \quad (43)$$

6.2 Uniform Bending



Moment of inertia:

$$I = \frac{bh^3}{12} \quad (44)$$

Curvature is moment over bending stiffness:

$$\kappa = \frac{1}{R} = \frac{M}{EI} \quad (45)$$

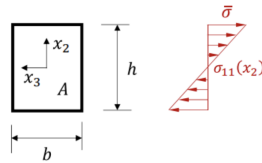
$$\varepsilon_{11} = \kappa x_2 \mathbf{e}_1 \otimes \mathbf{e}_1 \quad (46)$$

$$\varepsilon = \frac{1}{E} \left(-\nu \text{tr}(\sigma) \mathbf{I} + (1 + \nu) \sigma \right) \quad (47)$$

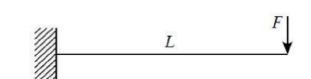
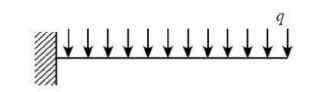
$$\sigma = E \cdot \kappa x_2 \mathbf{e}_1 \otimes \mathbf{e}_1 \quad (48)$$

Bend limit:

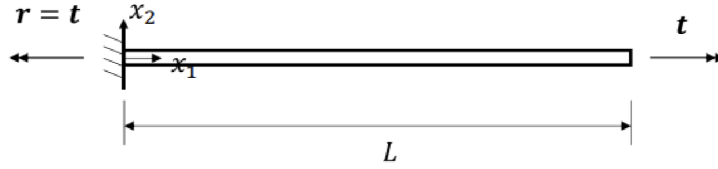
Since $\sigma_{11} = \kappa E x_2 = \frac{M}{I} x_2$, $\rightarrow \bar{\sigma} = \frac{Mh}{2I} \leq \sigma_Y$



$$M_{lim} = \frac{\sigma_Y b h^2}{6} \quad (49)$$

	$\theta(L)$	$w(L)$
$\frac{ML}{EI}$	$\frac{ML^2}{2EI}$	
	$\frac{FL^2}{2EI}$	$\frac{FL^3}{3EI}$
	$\frac{qL^3}{6EI}$	$\frac{qL^4}{8EI}$

6.3 Torsion



Cross sectional shear is total angle over length

$$\varphi = \frac{\phi}{l} \quad (50)$$

Hence rotation at x_1 is $\varphi \cdot x_1$.

$$u_2 = -\varphi \cdot x_1 x_3 \quad u_3 = \varphi \cdot x_1 x_2 \quad (51)$$

Resulting in:

$$[\varepsilon] = \begin{bmatrix} 0 & \frac{-\varphi}{2}x_3 & \frac{\varphi}{2}x_2 \\ \frac{-\varphi}{2}x_3 & 0 & 0 \\ \frac{\varphi}{2}x_2 & 0 & 0 \end{bmatrix} \quad (52)$$

$$[\sigma] = \begin{bmatrix} 0 & \mu \cdot \frac{-\varphi}{2}x_3 & \mu \cdot \frac{\varphi}{2}x_2 \\ \mu \cdot \frac{-\varphi}{2}x_3 & 0 & 0 \\ \mu \cdot \frac{\varphi}{2}x_2 & 0 & 0 \end{bmatrix} \quad (53)$$

Torque:

$$t = \int_A x_2 \sigma_{13} + x_3 \sigma_{12} dA = \varphi \cdot \mu I_p \quad (54)$$

(with $I_p = \frac{1}{2}\pi R^4$)

7 Elastoplasticity

Internally stored energy + applied/dissipated is smaller than zero:

$$\int_v \frac{d\psi}{dt} dV - \int_v P_{ext}(\dot{u}) dV = \int_v \frac{d\psi}{dt} dV - \int_v \sigma : \dot{\varepsilon} dV \leq 0 \quad (55)$$

Replace $\frac{d\psi}{dt}$ with derivative w.r.t. its inputs. (ex. $\frac{d\psi}{dt} = \frac{d\psi}{d\varepsilon} \cdot \dot{\varepsilon}$ (might require chain rule)).

$$(\psi = \psi(\varepsilon_e, \alpha) \rightarrow \frac{d\psi}{dt} = \frac{d\psi}{d\varepsilon_e} \cdot \dot{\varepsilon}_e + \frac{d\psi}{d\alpha} \cdot \dot{\alpha} = \frac{d\psi}{d\varepsilon_e} \cdot \dot{\varepsilon}_e + 0)$$

Dissipation (energy leaving per volume unit per time unit) (from definition of internal work) (Can often use $\sigma_{vm} = \sigma_Y$):

$$D = \sigma : \dot{\varepsilon}_p = \sigma_{vm} \cdot \dot{\varepsilon}_{eq} (\geq 0) \quad (56)$$

$$\sigma = \frac{\partial \psi}{\partial \varepsilon_e} \quad (57)$$

Von Mises equivalent stress (J2):

$$\sigma_{vm} = \sqrt{\frac{3}{2} \sigma^d : \sigma^d} \quad (58)$$

Von Mises strain:

$$\varepsilon_{eq} = \sqrt{\frac{2}{3} \varepsilon_p : \varepsilon_p} \quad (59)$$

Von Mises criterion: (with σ_i being principal stress (eigenvalue, of σ))

$$f(\sigma) = (\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2 - 2\sigma_Y^2 = 0 \quad (60)$$

Tresca criterion: (with σ_i being principal stress (eigenvalue, of σ))

$$f(\sigma) = \max(|\sigma_1 - \sigma_2|, |\sigma_1 - \sigma_3|, |\sigma_2 - \sigma_3|) - \sigma_Y = 0 \quad (61)$$

1D criterion:

$$f(\sigma) = |\sigma| - \sigma_Y (-\pi) = 0 \quad (62)$$

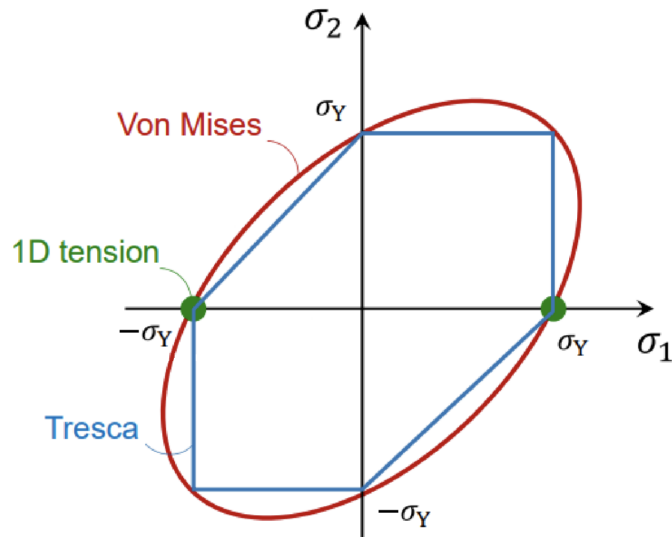


Figure 1: border defines yield surface, interior defines admissibility domain

Stress (σ) is dependent upon elastic strain (ε_e). Plastic deformation (π) is dependent upon plastic strain (ε_p) ($\dot{\lambda} = \dot{\gamma} = \dot{\alpha} = |\dot{\varepsilon}_p|$).

$$\dot{\varepsilon}_p = \dot{\gamma} R(\sigma) = \dot{\varepsilon}_{eq} \frac{f(\sigma)}{\sigma} \quad (63)$$

$$\psi(\varepsilon_e, \alpha) = \frac{1}{2} E \varepsilon_e^2 + \frac{1}{2} K \alpha^2 \quad (64)$$

$$\pi = -K \cdot \alpha \quad (65)$$

$$\sigma = \frac{\partial \psi}{\partial \varepsilon_e} = E \cdot \varepsilon_e = E(\varepsilon - \varepsilon_p) \quad (66)$$

7.1 Common hardening laws (J2)

Linear hardening:

$$\sigma_Y = \sigma_{Y0} + H \varepsilon_{eq}^p \quad (67)$$

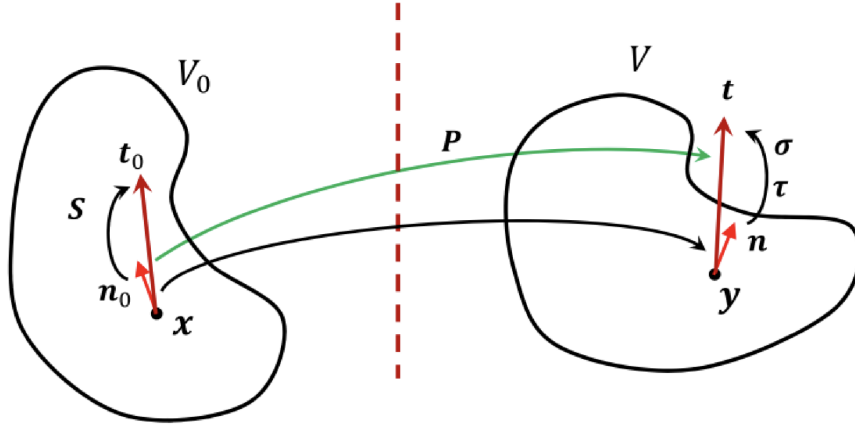
Ludwik law:

$$\sigma_Y = \sigma_{Y0} \left[1 + \left(\frac{\varepsilon_{eq}^p}{\varepsilon_{Y0}} \right)^n \right] \quad n \in [0, 1] \quad (68)$$

Ramberg-Osgood:

$$\varepsilon_{eq}^p = \frac{\sigma_Y}{E} \left[1 + \alpha \left(\frac{\sigma_Y}{\sigma_{Y0}} \right)^{m-1} \right] \quad m \geq 0 \quad (69)$$

8 Equilibrium at large deformations



First Piola-Kirchhoff stress tensor:

$$\mathbf{P} := \det(\mathbf{F}) \sigma \cdot \mathbf{F}^{-T} = \mathbf{F} \cdot \mathbf{S} \quad (70)$$

Second Piola-Kirchhoff stress tensor:

$$\mathbf{S} := \mathbf{F}^{-1} \cdot \mathbf{P} \quad (71)$$

Kirchhoff stress tensor:

$$\tau = \det(\mathbf{F}) \sigma = \mathbf{P} \mathbf{F}^T \quad (72)$$

$$\operatorname{div}(\mathbf{P}) + \mathbf{b}_0 = 0 \quad (73)$$

Balance of moments:

$$\mathbf{F} \cdot \mathbf{P}^T = (\mathbf{F} \cdot \mathbf{P}^T)^T \quad (74)$$

(Internal) virtual work:

$$\delta W_{int} = \int_V \mathbf{P} : \mathbf{F} dV (= \int_V P_{iA} F_{iA} dV) \quad (75)$$

Internally stored energy + applied/dissipated is smaller than zero:

$$\int_{v_0} \frac{d\psi}{dt} - \mathbf{P} : \dot{\mathbf{F}} dV_0 \leq 0 \quad \Longleftrightarrow \quad \frac{d\psi}{dt} - \mathbf{P} : \dot{\mathbf{F}} \leq 0 \quad (76)$$

8.1 Hyperelastic materials

- Define the kinematic variables $\rightarrow$ what ψ depends on;
- Derive the constitutive restrictions;
- Assign the specific Helmolzt free energy.
- Specify admissibility domain.
- Determine incremental constitutive equations

Kinematic Variables:

$$\psi = \psi(\mathbf{F}) = \psi(\mathbf{C}) \quad (77)$$

Constitutive restrictions:

$$\frac{d\psi}{dt} - \mathbf{P} : \dot{\mathbf{F}} \leq 0 \quad \Longleftrightarrow \quad \frac{\partial \psi}{\partial \mathbf{F}} : \dot{\mathbf{F}} - \mathbf{P} : \dot{\mathbf{F}} \leq 0 \quad \Longleftrightarrow \quad \left(\frac{\partial \psi}{\partial \mathbf{F}} - \mathbf{P} \right) : \dot{\mathbf{F}} \leq 0 \quad \Longleftrightarrow \quad \mathbf{P} = \frac{\partial \psi}{\partial \mathbf{F}} = 2\mathbf{F} \frac{\partial \psi}{\partial \mathbf{C}} \quad (78)$$

$$\mathbf{S} = \mathbf{F}^{-1} \cdot \mathbf{P} = \mathbf{F}^{-1} \cdot \left(2\mathbf{F} \frac{\partial \psi}{\partial \mathbf{C}} \right) = 2 \frac{\partial \psi}{\partial \mathbf{C}} = \frac{\partial \psi}{\partial \mathbf{E}} \quad (79)$$

(since $\mathbf{E} = \frac{1}{2}(\mathbf{C} - \mathbf{I})$)

$$\boldsymbol{\tau} = \mathbf{P} \cdot \mathbf{F}^T = \mathbf{F} \cdot \frac{\partial \psi}{\partial \mathbf{C}} \cdot \mathbf{F}^T \quad (80)$$

Helmolzt free energy: Must satisfy:

- $\psi \rightarrow \infty$, if $\det(\mathbf{F}) \rightarrow 0$
- Hooke's law is recovered for small deformations $\sigma = \mathbb{C} : \varepsilon$
- $\psi = 0$ if $\mathbf{F} = \mathbf{I}$

$$\psi(\mathbf{C}) = \frac{1}{2} \mathbf{E} : \mathbb{C} : \mathbf{E} \rightarrow S = \mathbb{C} : \mathbf{E} \quad (81)$$

Saint-Venant

- for isotropic and anisotropic elasticity
- for large rotations and small strain(unphysical under large compression)

$$\mathbf{S} = \lambda \text{tr}(\mathbf{E})\mathbf{I} + 2\mu\mathbf{E} \quad (82)$$

Mooney-Rivlin

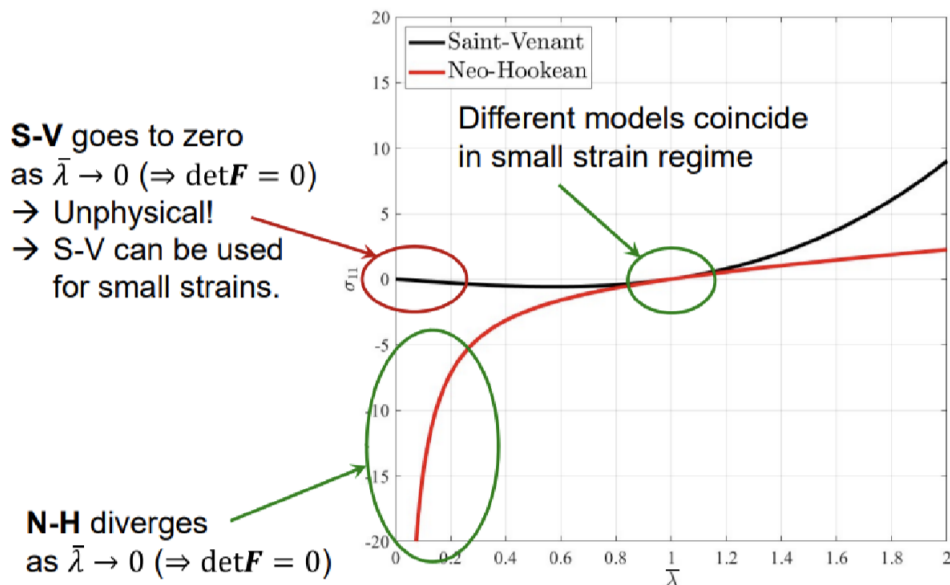
- for isotropic elasticity
- for large strains and rotations
- for **in**compressible materials (rubbers)

$$\mathbf{S} = (\mu_1 - \mu_2 \text{tr}(\mathbf{C}))\mathbf{I} + \mu_2\mathbf{C} \quad (83)$$

Neo-Hookean

- for isotropic elasticity
- for large strains and rotations
- for compressible materials (rubbers)

$$\mathbf{S} = \mu(\mathbf{I} - \mathbf{C}^{-1}) + \frac{\lambda}{2}(\det(\mathbf{F})^2 - 1)\mathbf{C}^{-1} \quad (84)$$



9 Polycrystal plasticity

$[abc]$: specific direction

$\langle abc \rangle$: family of directions

(abc) : specific plane

$\{abc\}$: family of planes

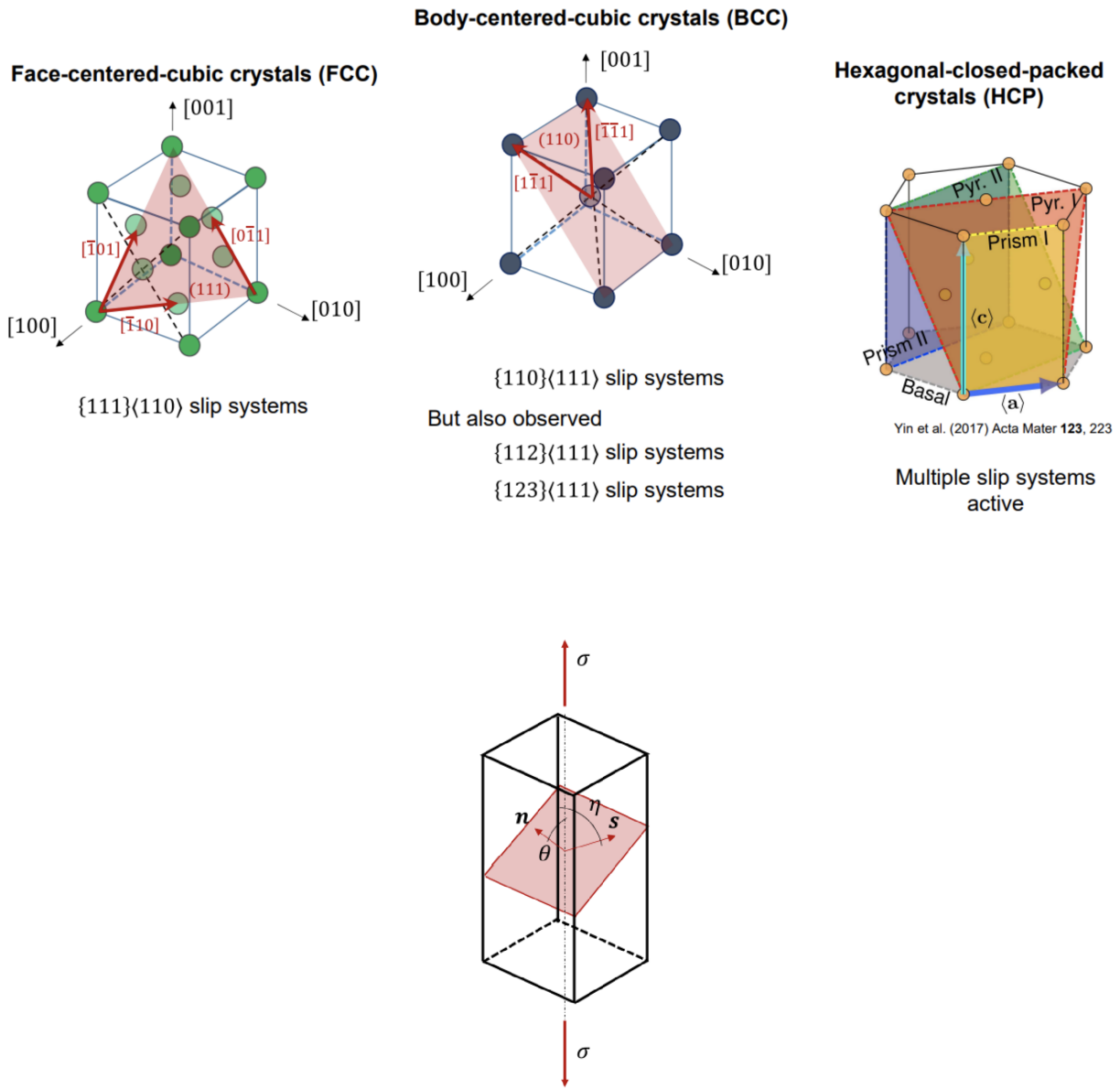


Figure 2: Slip trace due to shear along slip direction

Calculation of shear stress (τ): (*Schmid's law*)

$$\tau = \sigma \cos(\theta) \cos(\eta) \quad (85)$$

$$\sigma_Y = M \cdot \tau_Y \quad M = 3.067 \quad (86)$$

10 Crystal plasticity

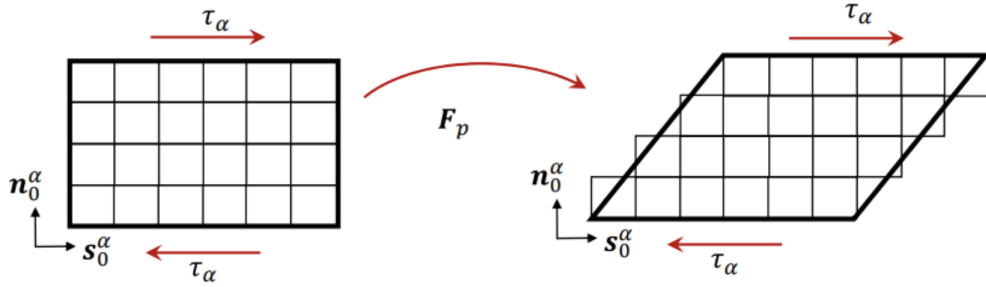


Figure 3: Continuum model

Schmid tensor (shows direction of shear) (slip direction: $\mathbf{s}_0^\alpha$, slip plane normal: $\mathbf{n}_0^\alpha$) :

$$\mathbf{P}_0^\alpha = \mathbf{s}_0^\alpha \otimes \mathbf{n}_0^\alpha \quad (87)$$

Shear stress on α^{th} shear system:

$$\tau_\alpha = \bar{\mathbf{M}} : \mathbf{P}_0^\alpha \quad (88)$$

$\mathbf{L}_p$ shows the direction of plastic deformation:

$$\mathbf{L}_p = \dot{\lambda} \mathbf{R}(\bar{\mathbf{M}}) = \dot{\mathbf{F}}_p \cdot \mathbf{F}_p^{-1} = \sum_{\alpha=1}^{N_s} \dot{\gamma}_\alpha \cdot \mathbf{P}_0^\alpha \quad (89)$$

Plastic velocity gradient:

$$d\mathbf{l}_i = \dot{\mathbf{F}}_p \cdot \mathbf{F}_p^{-1} \cdot d\mathbf{l}_i = \mathbf{L}_p \cdot d\mathbf{l}_i \quad (90)$$

Since $\psi = \psi(\mathbf{F}_e) = \psi(\mathbf{F} \cdot \mathbf{F}_p^{-1})$:

$$\mathbf{P} = \frac{\partial \psi}{\partial \mathbf{F}_e} \cdot \mathbf{F}_p^{-T} \quad (91)$$

Dissipation:

$$D = \mathbf{P} : \mathbf{F}_e \cdot \dot{\mathbf{F}}_p = (\mathbf{C}_e \cdot \bar{\mathbf{S}}) : \mathbf{L}_p \geq 0 \quad (92)$$

Rate dependence:

$$\frac{\dot{\gamma}_\alpha}{\dot{\gamma}_0} = \left(\frac{\tau_\alpha}{\tau_{\alpha,Y}} \right)^{\frac{1}{m}} \quad (93)$$

10.1 Hardening

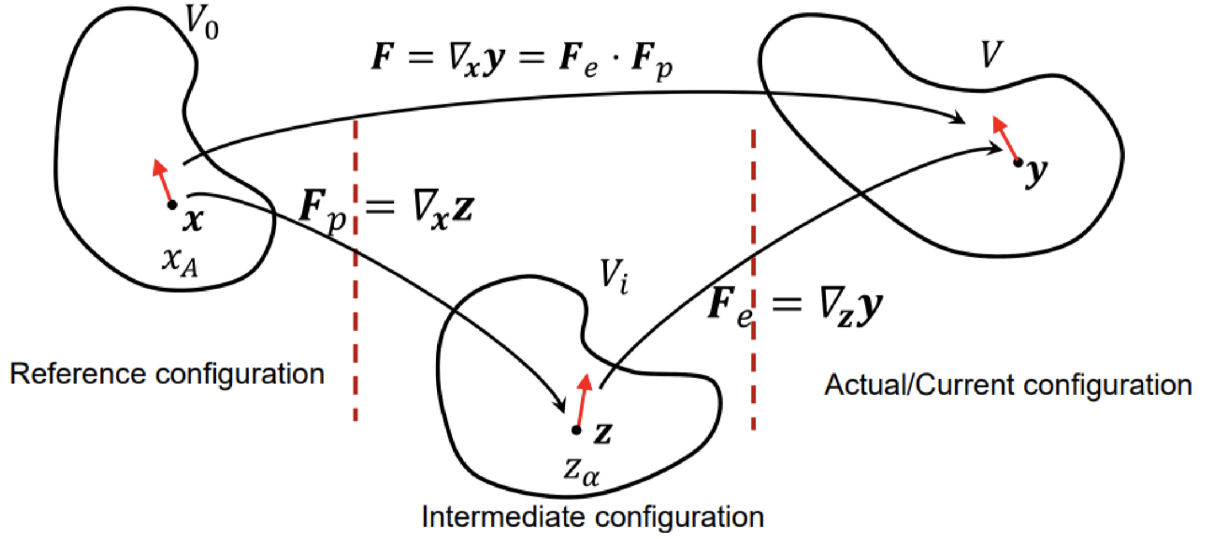
$$\tau_{\alpha,Y} = \tau_{\alpha,0} + \int_0^t \dot{g}_\alpha(\tau) d\tau \quad (94)$$

$$\dot{g}_\alpha(\tau) = h_{\alpha\beta} |\dot{\gamma}_\beta| \quad (95)$$

Hardening matrix:

$$h_{\alpha\beta} = h_0 \left(1 - \frac{\tau_{\beta,\gamma}}{\tau_\infty} \right)^a q_{\alpha\beta} \quad (96)$$

11 Elastoplasticity at large deformations



Total deformation is found using chain rule:

$$\nabla_{\mathbf{x}} \mathbf{z} = \frac{\partial \mathbf{z}(\mathbf{y}(\mathbf{x}))}{\partial \mathbf{y}} \cdot \frac{\partial \mathbf{y}}{\partial \mathbf{x}} = \nabla_{\mathbf{y}} \mathbf{z} \cdot \nabla_{\mathbf{x}} \mathbf{y} \quad (97)$$

$\mathbf{F}_p(t)$ is history dependent: $\dot{\mathbf{L}}_p$ explains in which direction $\mathbf{F}_p$ is changing.
Dissipation:

$$\mathbf{D} = (\mathbf{C}_e \cdot \bar{\mathbf{S}}) : \mathbf{L}_p = \mathbf{P} : \mathbf{F}_e \cdot \dot{\mathbf{F}}_p \geq 0 \quad (98)$$

$$\bar{\mathbf{S}} = \mathbf{F}_e^{-1} \cdot \mathbf{P} \cdot \mathbf{F}_p^T \quad (99)$$

Driving/Mandel stress (acting on intermediate configuration):

$$\bar{\mathbf{M}} = \mathbf{C}_e \cdot \bar{\mathbf{S}} \quad (100)$$

Von Mises equivalent stress:

$$\sigma_{vm} = \sqrt{\frac{3}{2} \bar{\mathbf{M}}^d : \bar{\mathbf{M}}^d} \quad (101)$$

$$f = \bar{M}_{eq}^d - \bar{M}_Y \leq 0 \quad (102)$$

Rate dependent flow:

$$\frac{\dot{\lambda}}{\dot{\lambda}_0} = \left(\frac{\bar{M}_{eq}^d}{\bar{M}_Y} \right)^M \quad (103)$$

Flow direction:

$$\mathbf{R} = \sqrt{\frac{3}{2}} \frac{\bar{\mathbf{M}}^d}{\bar{M}_Y} \quad (104)$$

$$\mathbf{P} = \frac{\partial \psi}{\partial \mathbf{F}_e} \mathbf{F}_p^{-T} \iff P_{iA} = \frac{\partial \psi}{\partial F_{i\alpha}^e} F_{\alpha A}^{p-1} \quad (105)$$

(if no plasticity, $\mathbf{F}_p = \mathbf{I}$)